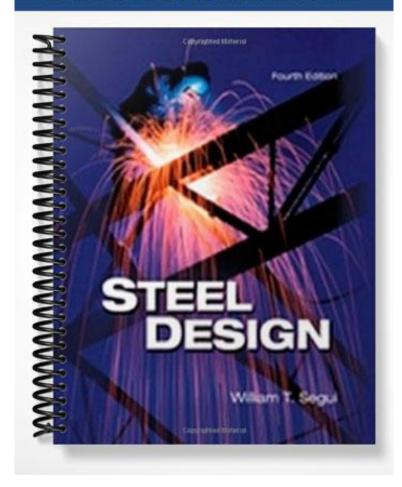
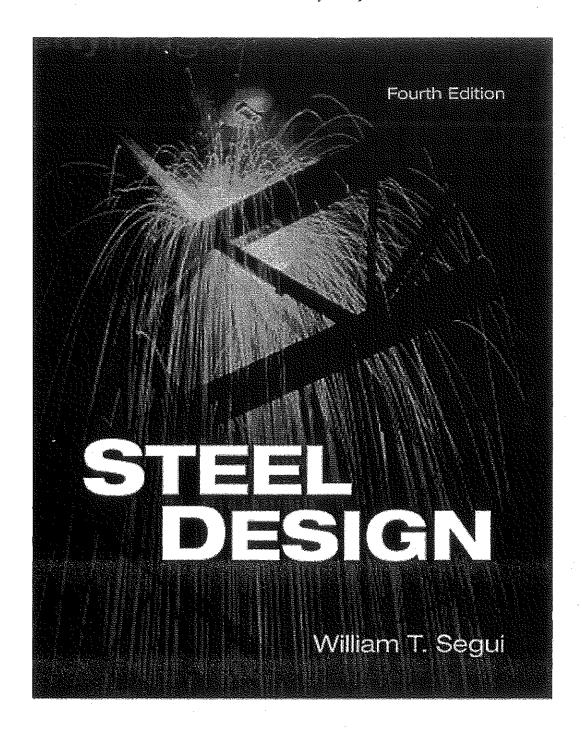
SOLUTIONS MANUAL



INSTRUCTOR'S SOLUTIONS MANUAL

to accompany







COPYRIGHT © 2007 by Thomson Engineering, a division of Thomson Learning Inc. Thomson Engineering is a registered trademark used herein under license.

For more information contact Nelson, 1120 Birchmount Road, Scarborough, Ontario M1K 5G4. Or you can visit our Internet site at www.engineering.thomsonlearning.com.

ALL RIGHTS RESERVED. No part of this work covered by the copyright hereon may be reproduced or used in any form or by any means—graphic, electronic, or mechanical, including photocopying, recording, taping, web distribution or information storage and retrieval systems—without the written permission of the publisher.

PREFACE

This instructor's manual contains solutions to the problems in Chapters 1-10 of *Steel Design*, 4th edition. Solutions to all problems are given, with the following exceptions: In Chapter 8 some of the ASD solutions are not given. In Chapters 9 and 10, only solutions to the problems whose answers appear in the Answers to Selected Problems section of the textbook are given.

In general, intermediate results to be used in subsequent calculations were recorded to four significant figures, and final results were rounded to three significant figures. Rounding was done according to the following practice: when the first digit to be dropped is less than 5, do not round up; when the first digit to be dropped is equal or greater than 5, add 1 to the last digit retained.

Students following these guidelines should be able to reproduce the numerical results given. However, the precision of the results could depend on the grouping of computations and on whether intermediate values are retained in the calculator between steps.

In many cases, there will be more than one acceptable solution to a design problem; therefore, the solutions given for design problems should be used only as a guide in grading homework.

I would appreciate learning of any errors you may discover. You can contact me at wsegui@memphis.edu. A list of errors and corrections will be maintained at

http://www.ce.memphis.edu/segui/errata.html.

In some instances, there are inconsistencies between dimensions and properties given in the *Manual* and values given in the database on the Companion CD. When I encountered a discrepancy, I used the value given in the *Manual*.

William T. Segui November 4, 2006



CONTENTS

Chapter 1.	Introduction	1-1
Chapter 2.	Concepts in Structural Steel Design	2-1
Chapter 3.	Tension Members	3-1
Chapter 4.	Compression Members	4-1
Chapter 5.	Beams	5-1
Chapter 6.	Beam-Columns	6-1
Chapter 7.	Simple Connections	7-1
Chapter 8.	Eccentric Connections	8-1
Chapter 9.	Composite Construction	9-1
Chapter 10	Plate Girders	10-1



CHAPTER 1 - INTRODUCTION

1.5-1

(a)
$$A = \pi (0.550)^2 / 4 = 0.2376 \text{ in.}^2$$

 $F_u = \frac{P_u}{A} = \frac{28,500}{0.2376} = 120,000 \text{ psi} = 120 \text{ ksi}$

 $F_u = 120 \text{ ksi}$

(b)
$$e = \frac{2.300 - 2.030}{2.030} \times 100 = 13.3\%$$

e = 13.3%

(c)
$$A_f = \pi (0.430)^2 / 4 = 0.1452 \text{ in.}^2$$

$$Change = \frac{A_f - A_o}{A} \times 100 = \frac{0.1452 - 0.2376}{0.2376} \times 100 = -38.9\%$$

Reduction = 38.9%

1.5-2

$$A = \pi (0.5)^{2} / 4 = 0.1963 \text{ in.}^{2}$$

$$f = \frac{P}{A} = \frac{13.5}{0.1963} = 68.77 \text{ ksi}$$

$$\varepsilon = \frac{\Delta L}{L} = \frac{4.66 \times 10^{-3}}{2} = 2.33 \times 10^{-3}$$

$$E = \frac{f}{\varepsilon} = \frac{68.77}{2.33 \times 10^{-3}} = 29,500 \text{ ksi}$$

E = 29,500 ksi

1.5-3

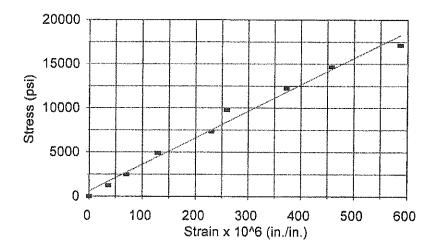
$$A = \pi (0.510)^2 / 4 = 0.2043 \text{ in.}^2$$

For $P = 250 \text{ lb}$, $f = \frac{P}{A} = \frac{250}{0.2043} = 1224 \text{ psi}$

Spreadsheet results:

(a)	Load	Stress	Strain x 10 ⁶
	(lb)	(psi)	(in./in.)
	0	0	0
	250	1224	37.1
	500	2447	70.3
	1000	4895	129.1
	1500	7342	230.1
	2000	9790	259.4
	2500	12237	372.4
	3000	14684	457.7
	3500	17132	586.5

(b)



$$E = \text{slope} = 30,100 \text{ ksi}$$

1.5-4

$$A = \pi (0.5)^2 / 4 = 0.1963 \text{ in.}^2$$

$$E = \frac{f}{\varepsilon} = \frac{P/A}{\Delta L/L} = \frac{P}{\Delta L} \cdot \frac{L}{A} = 1392(4/0.1963) = 28,400 \text{ ksi}$$

E = 28,400 ksi

1.5-5

$$A = \pi (3/8)^2 / 4 = 0.1104 \text{ in.}^2$$

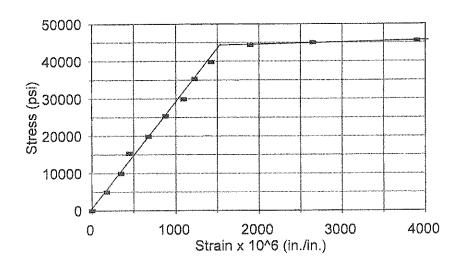
For $P = 550 \text{ lb}$, $f = \frac{P}{A} = \frac{550}{0.1104} = 4982 \text{ psi}$
 $\varepsilon = \frac{\Delta L}{L} = \frac{350 \times 10^{-6}}{2} = 175 \times 10^{-6}$

Spreadsheet results:

(a)

Load	Elongation x 10 ⁶	Stress	Strain x 10 ⁶
(lb)	(in.)	(psi)	(in./in.)
0	0	0	0
550	350	4982	175
1100	700	9964	350
1700	900	15399	450
2200	1350	19928	675
2800	1760	25362	880
3300	2200	29891	1100
3900	2460	35326	1230
4400	2860	39855	1430
4900	3800	44384	1900
4970	5300	45018	2650
5025	7800	45516	3900

(b)



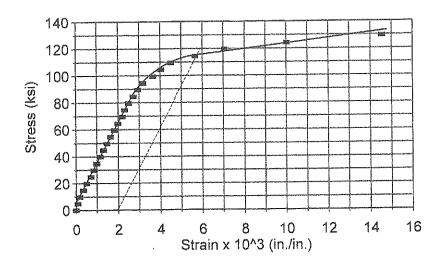
(c)
$$E = \text{slope} = \frac{15,000}{500 \times 10^{-6}} = 30,000,000 \text{ psi}$$

$$E = 30,000,000 \text{ psi}$$
 (d)
$$F_y \approx 44,000 \text{ psi}$$

1.5-6
Spreadsheet results:

(a) Elongation x 10³ Load Stress Strain x 103 (kips) (in.) (ksi) (in./in.) 0 0 0 0 1 0.16 4.973 0.080 2 3 0.352 9.945 0.176 0.706 14.92 0.353 4 1.012 19.89 0.506 5 1.434 24.86 0.717 6 7 1.712 29.84 0.856 1.986 34.81 0.993 8 2.286 39.78 1.143 9 2.612 44.75 1,306 10 2.938 49.73 1.469 11 3.274 54.70 1.637 12 3.632 59.67 1.816 13 3.976 64.64 1.988 14 4.386 69.62 2.193 15 4.64 74.59 2.320 16 4.988 79,56 2.494 17 5.432 84.54 2.716 18 5.862 89.51 2.931 19 6.362 94.48 3.181 20 7.304 99.45 3.652 21 8.072 104.4 4.036 22 9.044 109.4 4.522 23 11.31 114.4 5.655 24 14.12 119.3 7.060 25 20.044 124.3 10.02 26 129.3 29.106 14.55

(b)



(c)
$$E = \text{slope} = \frac{80 - 50}{0.0025 - 0.0015} = 30,000 \text{ ksi}$$

E = 30,000 ksi

(d)

 $F_{p\ell} \approx 85 \text{ ksi}$

(e)

 $F_{y} \approx 116 \text{ ksi}$

